

WATER TREATMENT METHOD

- The available water must be treated and purified before they can be supplied to the public for their domestic industrial or any other uses. The extent of treatment required to be given to the particular water depends upon characteristics and quality of the available water.

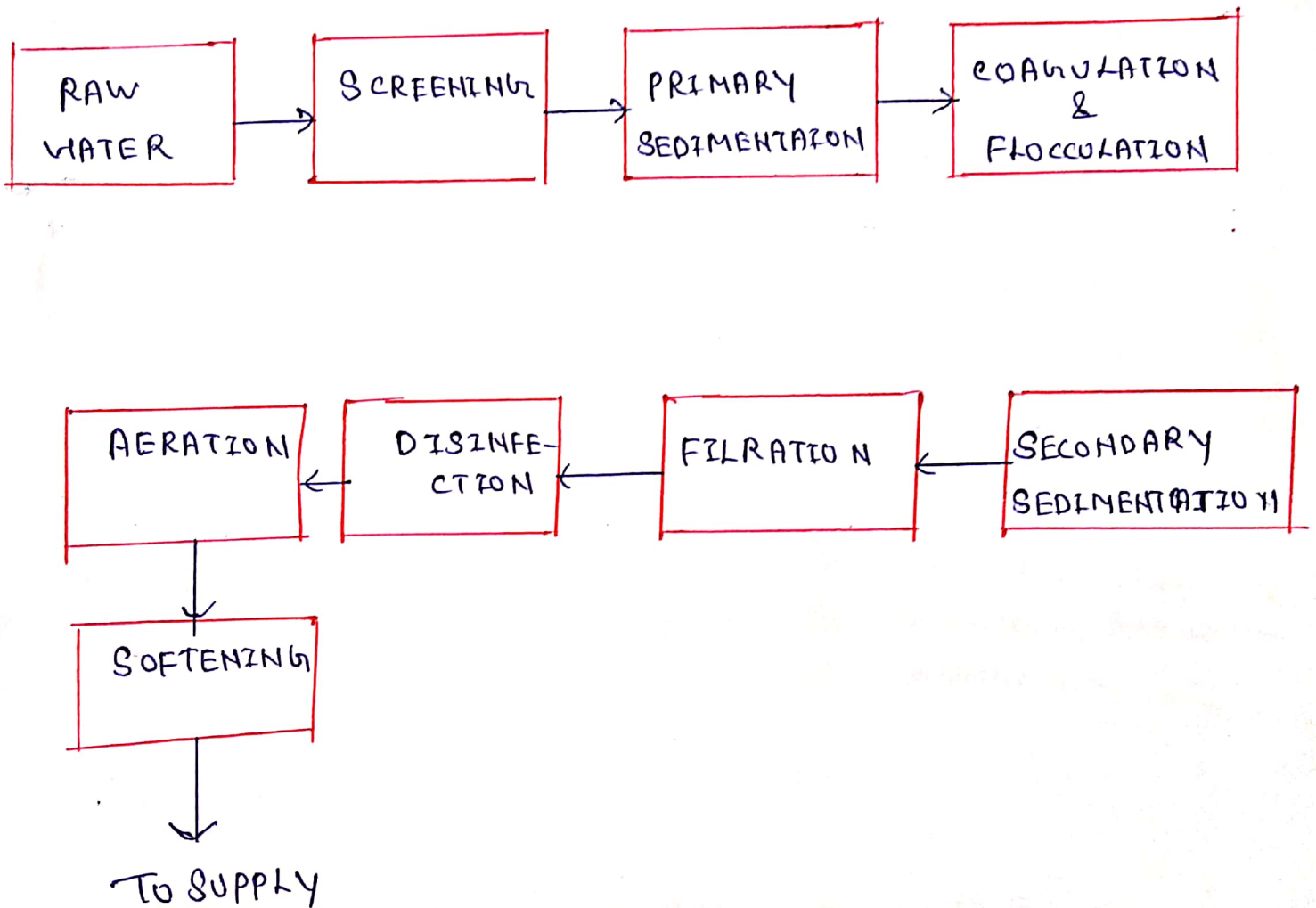


fig: Conventional water treatment plant

Purification of water \rightarrow

Screening \rightarrow Plain Sedimentation \rightarrow Sedimentation with Coagulation
 \rightarrow Filtration \rightarrow disinfection \rightarrow aeration \rightarrow Softening

① Screening \rightarrow

Inclination 45° to 60° with horizontal (To increase opening area so as to reduce velocity)

Velocity ≈ 0.8 to 1 m/s

① Bar Screens



Space 5 cm or above

Fine Screen.



2 cm to 5 cm .

Comminuters



cutting tools

Sedimentation is of two types

① intermittent (quiescent)

(ii) continuous

Principle based on Stokes law \rightarrow

$$V_s = \frac{g}{1.8} \frac{(S_s - 1) d^2}{\nu}$$

V_s = Terminal velocity

S_s = sp. gravity of sediment

ν = Kinematic viscosity of liquid

② Stokes equation

$$V_s = 418 (S_s - 1) d^2 \frac{(3T + 70)}{100} \text{ for streamline } (d < 0.1) \text{ mm}$$

③ Hazen equation

$$V_s = 418 (S_s - 1) d \frac{(3T + 70)}{100} \text{ for transition zone } (d \text{ b/w } 0.1 \text{ to } 1 \text{ mm})$$

④ Newton's equation

$$V_s = 1.8 \sqrt{gd(S_s - 1)} \text{ for free turbulent settling } (d > 1 \text{ mm})$$

- 70% of the suspended impurities present in water settle.

- Flow velocity = $\frac{Q}{B.H}$

M.H
7.2

- Settling velocity $V_s = \frac{Q}{B.L}$

Design criteria →

① over flow rate = $\frac{Q}{B.L}$ (discharge per unit flow surface area or plan area)
 = overflow rate or surface loading
 = 500-750 lit/hr/m² (plain sedimentation)
 = 1000-1250 lit/hr/m² (Coagulation sedimentation)

② Depth
 = 3 to 4.5 m, (1.8m and 60m)
 = L > 4B

③ Detention Period (time required to fill the tank)

(a) if there is no outflow, then time required to flow through the tank, t is

(a) $t = \frac{\text{Volume}}{\text{discharge}} = \frac{B.L.H}{Q}$ (for rectangular tank)

(b) $t = \frac{d^2(0.011d + 0.785H)}{Q}$ (for circular tank, cone shaped bottom with 1:1 slope)

$L > 4B$

$B = 10m$
 $> 12m$

Depth = 3 to 4.5 m
 (1.8m and 60m)

t = 4 to 8 hr (plain sedimentation)
 = 2 to 4 hr (Coagulation sedimentation)

Velocity = $\frac{L}{T}$

Displacement efficiency:-

Displacement $\eta_{ey} = \frac{\text{flowing through pd.}}{\text{detention pd.}} = 0.25 \text{ to } 5$

Q. water supply project, Population of town 50,000, with per capita demand = 150 lit/day

Solution

Assume

detention period = 4 hours.

depth = 3 m.

Velocity flow = 0.3 m/min

Per Capita demand = 150 lit/day

daily average demand of water = $150 \times 50,000 = 7,500,000$ lit/day

Peak demand = $1.5 \times 7,500,000 = 11,250,000$ lit/day

quantity of water to be detained = $\frac{11,250,000 \times 4}{24}$ lit/day
= $1,875 \text{ m}^3$

Capacity of tank = $1,875 \text{ m}^3$

$$\text{Velocity} = \frac{L}{T}$$

$$\begin{aligned} \text{Length of tank} &= \text{Velocity} \times \text{detention time} \\ &= 0.3 \times 4 \times 60 \\ &= 72 \text{ m} \end{aligned}$$

$$\text{Volume} = L \times B \times H$$

$$\text{Volume} = A \times H \times B = L \times A$$

$$\frac{\text{Volume}}{L} = A \Rightarrow \frac{1,875}{72} = 26.04 \text{ m}^2$$

$$\text{width of tank} = \frac{26.04}{3} = 8.6 \text{ m}$$

assuming free board as 0.5 m

$$\text{Actual depth} = \frac{8.6 + 0.5}{3} = 3 + 0.5 = 3.5 \text{ m}$$

considering 20% extra length for inlet and outlet work

$$\text{extra length} = 72 \times 0.2 = 14.4 \text{ m} = 14.5 \text{ m}$$

$$\text{net length} = 72 + 14.5 = 86.5 \text{ m}$$

therefore

$$\boxed{\text{dimension} = 86.5 \text{ m} \times 8.6 \times 3.5 \text{ m}}$$

Q. In a continuous flow settling tank 3.5 m deep and 65 m long. if the flow velocity of water offered is 1.22 cm/s. what size of the particle of specific gravity 2.65 may be effectively removed? Assume Temp 25°C and kinematic viscosity of water as 0.01 cm²/s

Solution

Depth of tank = 3.5 m (given)

out of this depth, 0.5 m can be assumed as free-board

Thus,

effective depth of tank or water depth = $H = 3.5 - 0.5 = 3 \text{ m}$

Flow velocity $V = 1.22 \text{ cm/sec}$

Length of tank $L = 65 \text{ m}$

Specific gr. of particles = 2.65

Temp $T = 25^\circ\text{C}$

kinematic viscosity $\nu = 0.01 \text{ cm}^2/\text{s}$

Let the particle size be 'd' and the settling velocity be V_s . we know that for a particle to be removed in the settling tank it must satisfy the relation

$$\frac{V}{V_s} = \frac{L}{H}$$

$$V_s = \frac{VH}{L} = \frac{1.22 \times 3}{65} = 0.0563 \text{ cm/s}$$

But settling velocity V_s as per Stoke's equation

$$V_s = \frac{g}{18} (S_s - 1) \frac{d^2}{\nu} \quad \text{for } d < 0.1 \text{ mm}$$

$$V_s = 418 (S_s - 1) d^2 \left(\frac{3T + 70}{100} \right) \quad \text{for } d < 0.1 \text{ mm}$$

$$V_s = \frac{381}{18} (2.65 - 1) \frac{d^2}{0.01}$$

$$0.0563 \times 18 \times 0.02 = 1.65 \times 981 \times d^2$$

$$\boxed{d = 0.002502} \text{ cm}$$

$d = 0.025 \text{ mm} (< 0.1 \text{ mm})$ Hence OK

$$V_s = 418 (2.65 - 1) \times d^2 \times \left(\frac{3 \times 28 + 70}{100} \right)$$

$$0.0563 \times 10 = 418 \times 1.65 \times d^2 \times \frac{145}{100}$$

$$\boxed{d = 0.024 \text{ mm}} \quad \leftarrow 0.1 \text{ mm Hence OK}$$

Thus, the particles of size 0.024mm and above shall be effectively removed.