

① Viscosity  $\rightarrow$  Pa·s to MKS  $\rightarrow \frac{\text{Pa} \cdot \text{s}}{10}$

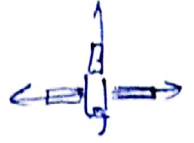
①

Pa·s to S.I  $\rightarrow \frac{\text{Pa} \cdot \text{s}}{10}$

② S.I unit of viscosity =  $\text{Ns/m}^2$

③ Pressure (P) b/w inside and outside of a liquid drop

$$P = \frac{4\sigma}{d}$$



$\rightarrow$  for liquid drop  $P = \frac{4\sigma}{d}$

$\rightarrow$  for soap bubble  $P = \frac{8\sigma}{d}$

$\rightarrow$  for liquid jet  $P = \frac{2\sigma}{d}$

④ Capillary rise or fall of a liquid

$$h = \frac{4\sigma \cos \theta}{\rho g d}$$

$\theta$  for water =  $0^\circ$

for mercury =  $128^\circ$

⑤ Absolute pressure  $\rightarrow$  absolute vacuum pressure is taken as datum

⑥ gauge pressure  $\rightarrow$  atm pressure is taken as datum

$$P_{\text{abs}} = P_{\text{atm}} + P_{\text{gauge}}$$

⑦ a single column manometer (or micromanometer) is used for measuring small pressures, where accuracy is required

\* Centre of Pressure is defined as the Point of application of the resultant Pressure. (2)

\* the depth of Centre of Pressure of an immersed surface from free surface of the liquid

$$h^* = \frac{I_G}{A\bar{h}} + \bar{h} \quad \text{for vertically immersed surface}$$

$$= \frac{I_G \sin^2 \theta}{A\bar{h}} + \bar{h} \quad \text{for inclined immersed surface}$$

\* the Centre of Pressure for a plane vertical surface lies at a depth of two-third the ht of the immersed surface

\* the inclination of the resultant force on curved surface with horizontal,  $\tan \theta = \frac{F_y}{F_x}$

\* the resultant force on a sluice gate  $F = F_1 - F_2$

\* the point through which force of buoyancy is supposed to act is called Centre of buoyancy

\* the point about which a body starts oscillating when the body is tilted is known as meta-centre

\* Condition of equilibrium of a floating and submerged body

Equilibrium	floating body	Submerged body
(i) Stable equilibrium	M is above G	B is above G
(ii) unstable equilibrium	M is below G	B is below G
(iii) Neutral equilibrium	M and G coincide	B and G coincide

\* discharge through a triangular notch or weir

(8)

$$Q = \frac{2}{3} C_{d1} \times L \times \sqrt{2g} \times H^{3/2} + \frac{8}{15} C_{d2} \times \tan \frac{\theta}{2} \times \sqrt{2g} \times H^{5/2}$$

\* the error in discharge due to the error in the measurement of head over a rectangular and triangular notch or weir is

$$\frac{dQ}{Q} = \frac{3}{2} \frac{dH}{dH}$$

--- for a rectangular notch or weir

$$\frac{dQ}{Q} = \frac{5}{2} \frac{dH}{H}$$

--- for a triangular notch or weir

\* Velocity of approach  $\Rightarrow$

the velocity with which the water approaches

the notch or weir is called the velocity of approach.

$$V_a = \frac{\text{Discharge over the notch or weir}}{\text{cross-sectional area of channel}}$$

\* the head due to velocity of approach is  $\Rightarrow h_a = \frac{V_a^2}{2g}$

\* discharge over a rectangular weir, with velocity of approach

$$Q = \frac{2}{3} C_d L \sqrt{2g} \left[ (H + h_a)^{3/2} - h_a^{3/2} \right]$$

\* Bazin's formula for discharge over a rectangular weir

$$Q = m L \sqrt{2g} H^{3/2} \rightarrow \text{without velocity of approach}$$

$$Q = m L \sqrt{2g} \left[ (H + h_a)^{3/2} \right] \text{ with velocity of approach}$$

$$m = \frac{2}{3} C_d = 0.405 + \frac{0.003}{H} \dots \text{without velocity of approach}$$

$$= 0.405 + \frac{0.003}{(H + h_a)} \dots \text{with velocity of approach}$$

\* Coefficient of discharge

$$C_d = \frac{Q}{Q_{th}} = \frac{\text{Actual Velocity} \times \text{Actual area}}{\text{Theoretical Velocity} \times \text{Theoretical area}} \quad (3)$$

$$C_d = C_v \times C_c$$

$$C_d = 0.61 \text{ to } 0.65$$

$$C_d = 0.62 \text{ taken}$$

\* the discharge through a large rectangular orifice is

$$Q = \frac{2}{3} C_d \times b \times \sqrt{2g} [H_2^{3/2} - H_1^{3/2}]$$

$H_1 = H_t$  of liquid above top edge of orifice  
 $H_2 = H_t$  of liquid above bottom edge of orifice

\* the discharge through fully submerged orifice

$$Q = C_d \times b \times (H_2 - H_1) \times \sqrt{2gH}$$

\* Discharge through Partially submerged orifice

$$Q = Q_1 + Q_2$$

$$= C_d b (H_2 - H_1) \times \sqrt{2gH} + \frac{2}{3} C_d b \sqrt{2g} [H_2^{3/2} - H_1^{3/2}]$$

\* A notch is a device used for measuring the rate of flow of a liquid through a small channel.

— A weir is a dam or masonry structure placed in the open channel over which the flow occurs

$$Q = \frac{2}{3} C_d \times L \times H^{3/2}$$

$C_d$  = Coefficient of discharge  
 $L$  = Length of notch or weir  
 $H$  = Head of water over the notch or weir

\* the discharge over a triangular notch or weir

$$Q = \frac{8}{15} \times C_d \tan \frac{\theta}{2} \times \sqrt{2g} \times H^{5/2}$$

$$* h = \left( \frac{P_1}{\rho g} + z_1 \right) - \left( \frac{P_2}{\rho g} + z_2 \right) = x \left[ 1 - \frac{S_l}{S_o} \right] \dots \quad (5)$$

[for inclined venturimeter in which differential manometer contains lighter liquid]

$x$  = difference in the reading of differential manometer

$S_h$  = sp. gr. of heavier liquid

$S_o$  = sp. gr. of flowing through venturimeter

$S_l$  = sp. gr. of lighter liquid

\* Orifice is a small opening on the side or at the bottom of a tank

- mouthpiece is a short length of pipe which is two or three times its diameter in length.

\* Hydraulic coefficient

- Coefficient of velocity ( $C_v$ ) = 
$$\frac{\text{Actual velocity of Jet at vena contracta}}{\text{Theoretical velocity}}$$

$$= \frac{V}{\sqrt{2gH}}$$

$$C_v = 0.95 \text{ to } 0.99$$

$$\boxed{C_v = 0.98} \leftarrow \text{Sharp edged orifices.}$$

- Coefficient of contraction ( $C_c$ ) = 
$$\frac{\text{area of Jet at vena contracta}}{\text{area of orifice}}$$

$$= \frac{a_c}{a}$$

$$C_c = 0.61 \text{ to } 0.69$$

$$\boxed{C_c = 0.64}$$

\* the time Period of oscillation or rolling of a floating body (3) (3)

$$T = 2\pi \sqrt{\frac{k^2}{GM \times g}}$$

$k$  = Radius of gyration

$GM$  = meta Centre height

$T$  = Time of one complete oscillation

\* the relation b/w tangential velocity and radius

$$\text{for forced motion, } v = \omega \times r$$

$$\text{for free motion, } v \times r = \text{constant}$$

\* for a forced motion flow in a closed tank

$$\text{fall of liquid level at centre} = \text{rise of liquid level at ends}$$

\* for a free motion flow the equation

$$\frac{P_1}{\rho g} + \frac{v_1^2}{2g} + z_1 = \frac{P_2}{\rho g} + \frac{v_2^2}{2g} + z_2$$

\* Bernoulli's equation for real fluids

$$\frac{P_1}{\rho g} + \frac{v_1^2}{2g} + z_1 = \frac{P_2}{\rho g} + \frac{v_2^2}{2g} + z_2 + h_L$$

$h_L$  = Loss of energy b/w section 1 and 2

\* value of  $h$  is given by differential U-tube manometer

$$h = x \left[ \frac{S_h}{S_o} - 1 \right] \dots \text{[when differential manometer contains heavier liquid]}$$

$$h = x \left[ 1 - \frac{S_l}{S_o} \right] \dots \text{[when differential manometer contains lighter liquid]}$$

$$h = \left( \frac{P_1}{\rho g} + z_1 \right) - \left( \frac{P_2}{\rho g} + z_2 \right) = x \left[ \frac{S_h}{S_o} - 1 \right] \text{ (for inclined venturimeter in which differential manometer contains heavier liquid)}$$

\* fluid characteristics like velocity, pressure, density etc, do not change at a point with respect to the time the fluid flow is called Steady flow

$$\left(\frac{dv}{dt}\right) = 0 \text{ for Steady flow}$$

$$\left(\frac{dv}{dt}\right) \neq 0 \text{ for unsteady flow}$$

\* if the velocity in a fluid flow does not change with respect to space (length of direction of flow), the flow is said to be uniform rather than non-uniform

$$\left(\frac{\partial v}{\partial s}\right) = 0 \text{ for uniform flow}$$

$$\left(\frac{\partial v}{\partial s}\right) \neq 0 \text{ for non-uniform flow}$$

\* Components of velocity in x, y, and z direction in terms of Velocity Potential ( $\phi$ ) are

$$\boxed{u = -\frac{\partial \phi}{\partial x}}, \quad \boxed{v = -\frac{\partial \phi}{\partial y}}, \quad \boxed{w = -\frac{\partial \phi}{\partial z}}$$

\* Stream function ( $\psi$ ) is defined only for two-dimensional flow. the velocity components in x and y direction in terms of stream function

$$\boxed{u = -\frac{\partial \psi}{\partial y}} \quad \boxed{v = \frac{\partial \psi}{\partial x}}$$

\* Angular deformation or Shear strain rate is given by as

$$\text{Shear strain rate} = \frac{1}{2} \left[ \frac{\partial v}{\partial x} + \frac{\partial u}{\partial y} \right]$$